Warsaw University of Technology

Institute of Aeronautics and Applied Mechanics

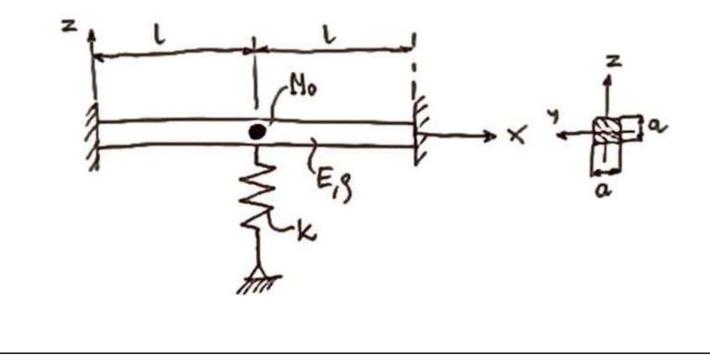
Finite element method 2 (FEM 2)

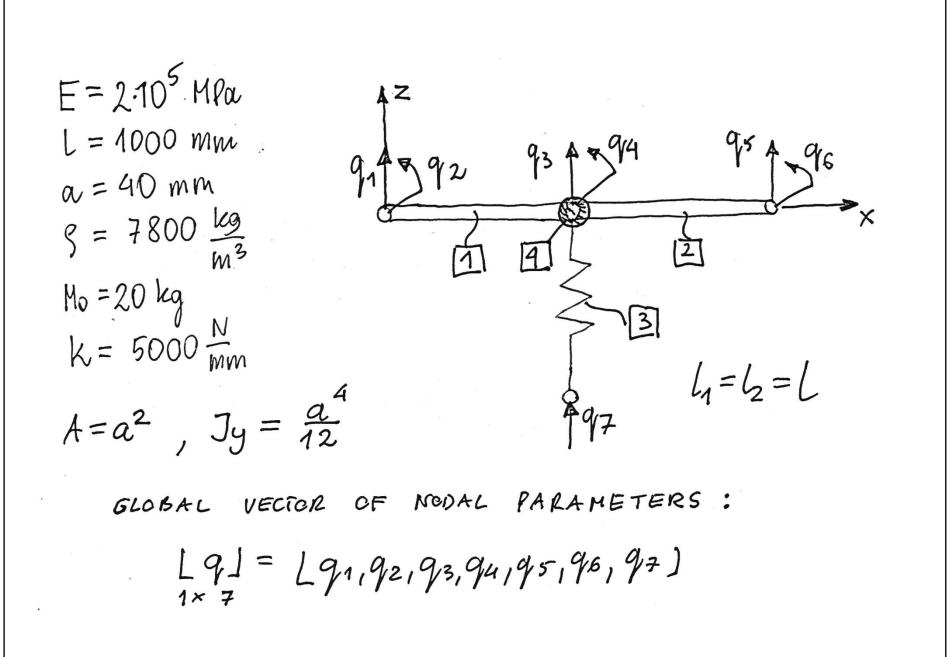
Example of modal analysis

11.2021

Example. Build a finite element model and write the set of equations for modal analysis of a 2D structure consisting of a beam, spring and mass M_0 . The beam is represented by 2 finite elements.

- a) Find natural frequencies (f_1 , f_2) and corresponding eigenvectors. Draw the mode shapes.
- b) Calculate the minimum value of spring stiffness, so that the mode shape without the translation of mass M_0 becomes the first vibration mode.





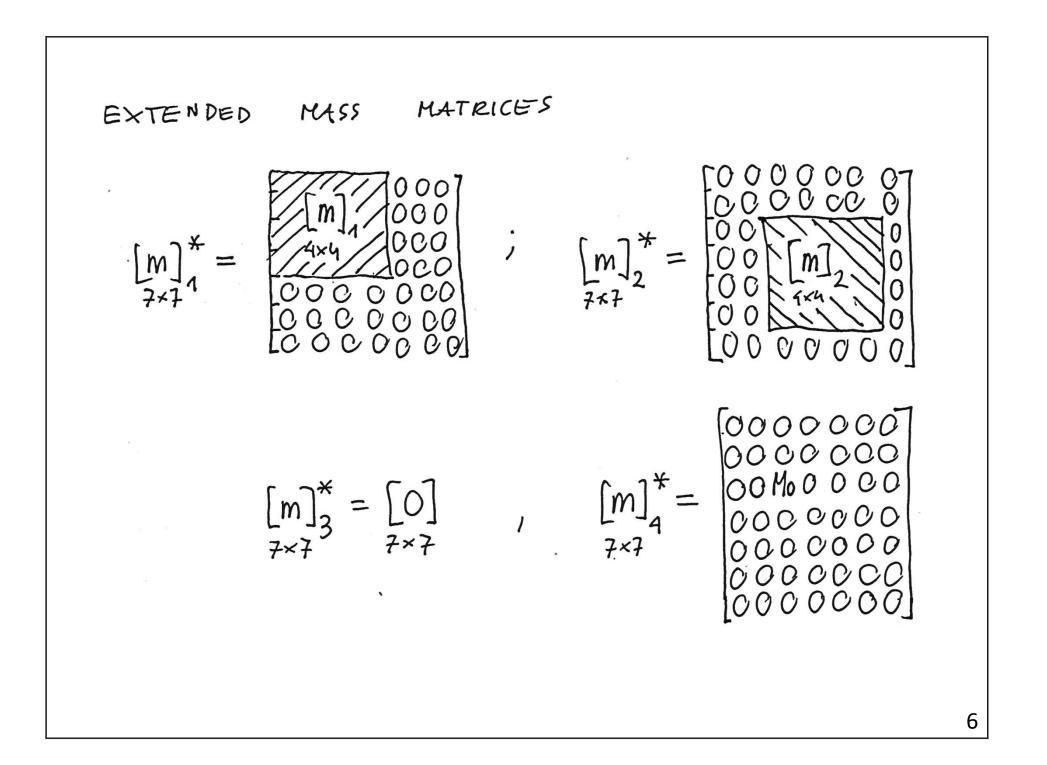
LOCAL STIPFNESS MATRICES :

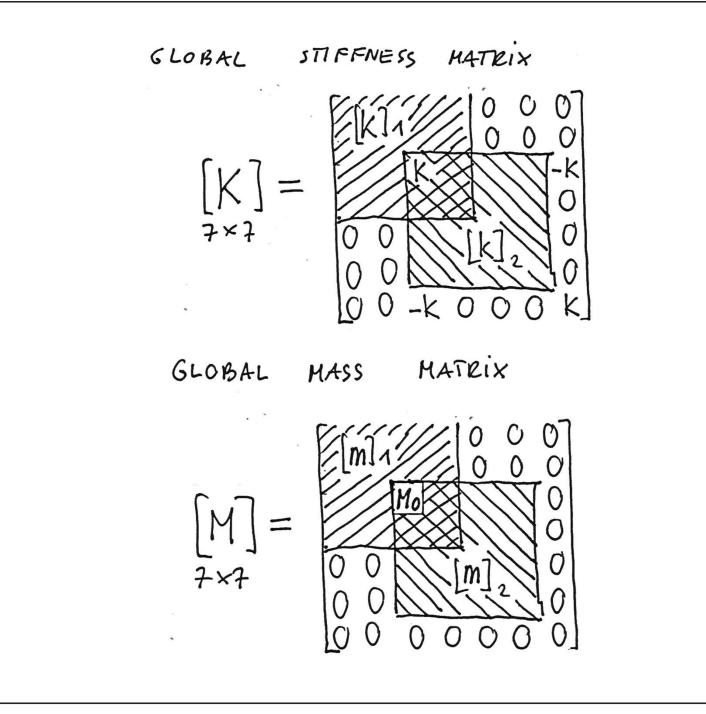
LOCAL MASS MATRICES :

$$\begin{bmatrix} m \end{bmatrix}_{4 \times 4} = \begin{bmatrix} m \end{bmatrix}_{2} = \frac{ga^{2}l}{420} \cdot \begin{bmatrix} 156 & 22l & 54 & -13l \\ 22l & 4l^{2} & 13l & -3l \\ 54 & 13l & 156 & -22l \\ -13l & -3l & -22l & 4l^{2} \end{bmatrix}$$

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 $\begin{bmatrix} m \\ 2 \times 2 \end{bmatrix}_{3} = \begin{bmatrix} 0 & 0 \\ 0 & 0 \end{bmatrix}, \quad \begin{bmatrix} m \\ (1 \times 1) \end{bmatrix}_{4} = \begin{bmatrix} M_{0} \\ (1 \times 1) \end{bmatrix}$





BOUNDARY CONDITIONS $q_1 = 0$, $q_2 = 0$, $q_5 = 0$, $q_6 = 0$, $q_7 = 0$ SET OF EQUATIONS : $\begin{bmatrix} k_{33-1} + k_{41-2} + k & k_{34-1} + k_{12-2} \\ k_{43-1} + k_{21-2} & k_{44-1} + k_{22-2} \end{bmatrix}$ $-\omega^{2} \left[\frac{M_{33_{-1}} + M_{11-2} + M_{0}}{M_{43_{-1}} + M_{21-2}} + \frac{M_{0}}{M_{44_{-1}} + M_{22_{-2}}} \right] \cdot \left\{ \begin{array}{c} q_{3} \\ q_{3} \\ q_{4} \\ q_{4}$

$$\begin{aligned} k_{33,1} + k_{41,2} + k &= \frac{6Ea^4}{6l^3} + \frac{6Ea^4}{6l^3} + k = \frac{2Ea^4}{l^3} + k \\ k_{34,1} + k_{42,2} &= -\frac{3l \cdot Ea^4}{6l^3} + \frac{3l \cdot Ea^4}{6l^3} = 0 = k_{43,1} + k_{21,2} \\ k_{4h,1} + k_{22,2} &= \frac{2l^2 \cdot Ea^4}{6l^3} + \frac{2l^2 Ea^4}{6l^3} = \frac{2Ea^4}{3l} \\ m_{33,1} + m_{41,2} + M_0 &= \frac{456 \cdot ga^2 l}{420} + \frac{456 \cdot ga^2 l}{420} + M_0 = \frac{26 \cdot ga^2 l}{35} + M_0 \\ m_{34,-1} + m_{42,2} &= -\frac{22l \cdot ga^2 l}{420} + \frac{22l \cdot ga^2 l}{420} = 0 = m_{43,-1} + m_{21,2} \\ m_{44,1} + m_{22,2} &= \frac{4l^2 \cdot ga^2 l}{420} + \frac{4l^2 \cdot ga^2 l}{420} = \frac{2 \cdot ga^2 l^3}{105} \end{aligned}$$

$$\begin{pmatrix} 2Ea^{4} + k & 0 \\ 0 & 2Ea^{4} \\ 0 & 3L \end{pmatrix} - \omega^{2} \begin{pmatrix} 26 ga^{2}L + No & 0 \\ 35 & +No & 0 \\ 0 & 2ga^{2}L^{3} \\ 0 & 705 \end{pmatrix} \cdot \begin{pmatrix} q_{3} \\ q_{4} \end{pmatrix} = \begin{cases} 0 \\ 0 \\ 0 \end{pmatrix}$$

$$det \begin{pmatrix} \Gamma_{X} \\ \Gamma_{XL} - \omega^{2} \begin{pmatrix} H \\ T \\ 2xL \end{pmatrix} = 0$$

$$Mo = 20 kg = 20 \frac{kgm}{s^{2}} \cdot \frac{s^{2}}{m} = 20 \frac{Ns^{2}}{1000mm} = 0.02 \frac{Ns^{2}}{mm} \quad f_{1} = 72.2 Hz$$

$$s = 7800 \frac{kg}{m^{3}} = 7800 \frac{kgm}{s^{2}} \cdot \frac{s^{2}}{mm^{3}} = 78.10 \frac{-9Ns^{2}}{mm^{3}}$$

$$\begin{cases} \left(\frac{2Ea4}{l^{3}}+k\right) - \omega^{2} \left(\frac{26ga^{2}l}{35}+N_{0}\right) \right\} \cdot \left(\frac{2Ea4}{3l} - \omega^{2} \cdot \frac{2ga^{2}l^{3}}{105}\right) = 0. \\ \downarrow & \omega_{1} = \sqrt{\frac{2Ea4}{l^{3}}+k} = \frac{1}{2ga^{2}l} + \frac{1}{2ga^{2}l} = \frac{1}{2ga^{2}l} + \frac{1}{2ga^{2}l} = \frac{1}{2ga^{2}l} = \frac{1}{105} = \frac{1}{105$$

EIGEN VECTORS

NEW CONSTANTS :

$$\Phi(\omega_i) = \frac{2Ea^4}{l^3} + k - \omega_i^2 \left(\frac{26ga^2l}{35} + M_0\right)$$

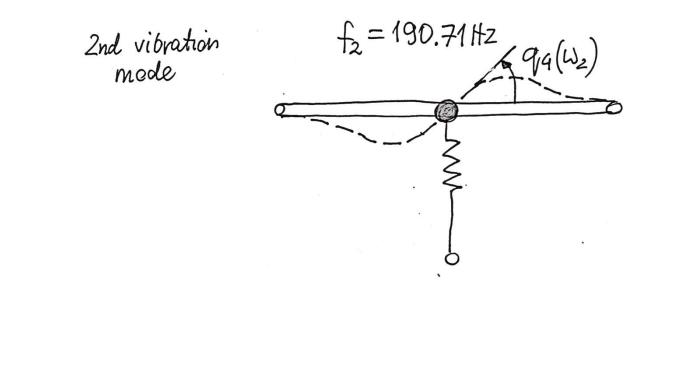
$$Y(\omega_i) = \frac{2\epsilon a}{3\iota} - \omega_i^2 \cdot \frac{25a}{105}$$

 $\begin{aligned} & \Phi(\omega_i) \cdot q_3(\omega_i) + D \cdot q_4(\omega_i) = 0 \\ & O \cdot q_3(\omega_i) + Y(\omega_i) \cdot q_4(\omega_i) = 0 \end{aligned}$

 $\underline{\Gamma} + \underline{\Gamma}$: $\Phi(\omega_i) \cdot q_3(\omega_i) + \Psi(\omega_i) \cdot q_4(\omega_i) = 0$ for ω_1 : $\overline{\phi}(\omega_n) = 0$, $Y(\omega_n) \neq 0$ $0 \cdot q_3(W_1) + \Psi(W_1) \cdot q_4(W_1) = 0$ Le any real value $L_P = 0$ $q_{3}(\omega_{1})$ 1st vibration mode $f_1 = 72.2 \text{ Hz}$

13

for ω_2 : $\overline{P}(\omega_2) \neq 0$, $\overline{Y}(\omega_2) = 0$ $\overline{P}(\omega_2) \cdot \underline{q_3}(\omega_2) + 0 \cdot \underline{q_4}(\omega_2) = 0$ = 0 $\overline{L_2}$ any real value



b) for W2 Mass Mo only rotates: $\tilde{\omega}_{1}^{2} > \omega_{2}^{2}$ $\frac{2 Ea^4}{l^3} + \tilde{k}$ $\frac{35 Ea^2}{35} + M_0$ $\frac{35 Ea^2}{35}$ $\tilde{k} > \frac{35Ea^2}{gl^4} \cdot \left(\frac{26ga^2l}{35} + M_0\right) - \frac{2Ea^4}{l^3}$ $\tilde{k} > 4.1.10^4 \frac{N}{mm}$ 15

if the moment of inertia is considered:

steel ball Mo $g = \frac{M_0}{V} = \frac{3M_0}{4\pi R^3} = R_0 = \frac{3}{4\pi R^3} = 85 \text{ mm}$

 $J_{o} = \frac{2}{5} M_{o} \cdot R^{2} = \frac{2}{5} \cdot 0.02 \frac{Ns^{2}}{mm} \cdot 85^{2} mm^{2} = 57.8 Ns^{2} mm$

fo'= 171 Hz

 $\tilde{k}' > \frac{2Ea^{4}}{\frac{3L}{2ga^{2}L^{3}}+J_{0}} \left(\frac{26ga^{2}L}{35}+N_{0}\right) - \frac{2Ea^{4}}{L^{3}}, \tilde{k}' > 3.28.10^{4} \frac{N}{mm}$